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Use of an external organic carbon source in the removal of nitrates in Bio-sand filters (BSF)

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### **ABSTRACT**

Bio-sand filters (BSF) are point of use (POU) potable water filtration systems commonly used in low-income communities at household level. The principle of operation is similar to that of a slow sand filter and the major difference is that they are operated intermittently at the point of use. It is one of the emerging low cost technologies which makes use of readily and locally available construction materials but is poor in the removal of nitrates. In order to enhance the removal of nitrates through de-nitrification, a modified bio-sand filter with ethanol as an external carbon source at C/N ratios of 1.1 and 1.8 was investigated. In the absence of an external carbon source, the nitrate removal efficiency was 32% whilst removal efficiencies at C/N ratios of 1.1 and 1.8 were 44% and 53% respectively. The inflow rate reduced significantly from an initial flow rate of 0.04m<sup>3</sup>/hr to 0.01m<sup>3</sup>/hr. The reduction in the inflow rate was mainly due to the growth of the biological layer on the filter media. The study showed that the use of an external carbon source like ethanol in biosand filtration enhances the removal of nitrates in potable water.

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## INTRODUCTION

Bios-sand filters are intermittent slow sand filters designed for household use and hence called point of use (POU) water filtration systems, with principal filtration mechanisms being physical, chemical and biological. The biological mechanisms take place at the top layer where a biological mat develops in the 50 to 100mm of the media. The biological layer acts both as a fine filter to remove small colloidal particles, dissolved impurities and at the same time immobilizes pathogens.

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Although bio-sand filters are now widely applied in the treatment of water at household level, few studies have been conducted on the removal of chemical contaminants. Current research in BSF has mainly focussed on the removal of pathogenic organisms like Escherichia coli (E. coli) and suspended solids. One chemical of major concern is nitrate-nitrogen contamination (NO<sub>3</sub>-N) in surface and ground water as it poses serious health problems (Almasiri and Kaluarachchi, 2007). Methaemoglobinemia in infancy is related to nitrate ingestion resulting in low oxygen in-intake and consequently causing death. Furthermore, presence of nitrates in drinking water results in the formation of nitrosomines in the stomach, which are carcinogenic. Nitrate poisoning has been reported in livestock when concentrations exceeded 100mg/l (Tredoux et al., 2000) and other problems related to nitrate in drinking water are well documented in literature (Moraes 1995; Fan and Steinberg, 1996; Lin et al., 2002; Forman 2004).

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Main sources of NO<sub>3</sub>-N in surface waters and groundwater aguifers include use of agricultural fertilizers, animal waste disposal, wastewater effluents from conventional and on-site sanitation facilities. Water supply from high nitrate concentration

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environments needs some form of treatment or dilution with low-nitrate content water. The current design of conventional biosand filters has been proved to be poor in the removal of nitrates (Heather et al., 2010; Mahlangu et al., 2011; Kennedy et al., 2012). Physical and chemical methods such as ion-exchange, reverse osmosis, electrodialysis, distillation, nanofiltration and activated carbon have been applied in the removal of nitrates from drinking water supplies. These methods are relatively expensive and show poor selectivity for nitrate removal with generation of brine, which is difficult to dispose (Moheseni et al., 2013). Hence there is need to explore alternative technologies like biological de-nitrification which has been proved to be efficient in complete nitrate elimination and has the advantage of producing a harmless by-product ( $N_2$ ). The pathway for nitrate removal by heterotrophic bacteria is:-nitrate  $\rightarrow$ nitrite $\rightarrow$  nitric oxide  $\rightarrow$  nitrous oxide  $\rightarrow$ gaseous diatomic nitrogen:-

$$NO_3^- \rightarrow NO_2^- \rightarrow NO \rightarrow N_2O \rightarrow N_2 \uparrow$$

The biological de-nitrification technology is based on the conventional theory, that carbon is the limiting factor in the efficiency of biological de-nitrification. Heterotrophs utilises carbon from organic compounds like sugars, organic acids and amino acids as source of electrons rather than from inorganic compounds like carbon dioxide as the case in autotrophic de-nitrification. Although autotrophic nitrate removal has the advantage of not requiring an organic carbon source, it is associated with slow growth rate of autotrophic bacteria and low nitrate removal rate.

Few studies have been conducted on the ability of the bio-sand filters in the removal of nitrates. In a study conducted in rural Cambodia by Heather et al. (2010), it was revealed that there was simultaneous nitrification and de-nitrification occurring in the bio-sand filters. However, about 85% of the biofilters under study did not meet the WHO guideline for NO<sub>3</sub>-N in the treated effluent. The study showed that de-nitrification was predominant when the inflow into the filter was from surface water, which could be due to the high organic carbon content. Kennedy et al. (2012) studied the effects of hydraulic loading on removal of nitrates in biosand filters and the overall nitrate removal efficiency was low (16%). Mahlangu et al. (2011) established that the conventional BSF and the modified BSF of zeolites (clinoptilote) have relatively low removal rates of nitrates (37%). On the same study, other types of biofilters which include ceramic candle and bucket filters had poor removal of nitrates ranging from 18% to 37%. On certain occasions, the effluent concentration of NO<sub>3</sub>-N was even higher than the unfiltered water and possibly due to desorption of previously adsorbed nitrates.

Most sources of drinking water lack sufficient quantities of organic carbon for cell growth and energy source. The organic carbon acts as both a source of cellular material for biological respiration and electron donor for dissimilatory nitrate reduction. Waters with low carbon content require an external carbon source for de-nitrification to take place under anoxic conditions and nitrate is converted to gaseous diatomic nitrogen.

A variety of external carbon sources like sucrose, ethanol, methanol and acetic acid have been applied in conventional slow sand filters to aid heterotrophic denitrification at C/N ratios ranging from 1 to 2.5. The studies have shown considerable improvement levels in the de-nitrification process due to the recorded high nitrate

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removal efficiencies of about 90% (Green et al., 1994). Gomes et al. (2000) assayed the influence of sucrose, ethanol, methanol and ethyl alcohol in nitrate reductase in contaminated groundwater and showed very high removals with effluent concentrations ranging from 0 to 5mg/l. Aslan and Cakici (2007), reported removal rate of 94% for nitrate in slow sand filters when acetic acid was used as a carbon source. Methanol is toxic due to some of the residual concentrations of carbonaceous compounds found in the effluent and produces an excessive growth of biomass. Sucrose and glucose have a tendency to form a biomass which increases turbidity in the final effluent. Acetic acid and ethanol are considered to be the most suitable carbon sources in removal of nitrate and no limits have been set in potable water. They are also cheaper, a concept inherent with the use of bio-sand filtration technology.

However, heterotrophic de-nitrification has not been investigated in bio-sand filters except in the conventional slow sand filters. The aim of this study was to investigate the removal of NO<sub>3</sub>-N in biosand filters with ethanol as a carbon source and to establish the optimum Carbon to Nitrate (C/N) ratio for microbial activity which achieves maximum removal with minimum excess carbon in the effluent.

# **MATERIALS AND METHODS**

Two bio-sand filters were investigated at household level: - one with an external carbon source (BSFC) to enhance the de-nitrification process at C/N ratios of 1.1 and 1.8, and the other one without a carbon source (BSF). The two bio-sand filters were dosed with known concentrations of ammonium nitrate which was the source of nitrate.

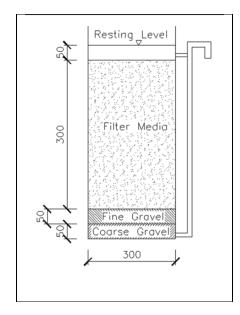


Figure 1 Schematic representation of the bio-sand filter (dimensions in mm).

### Filter construction

The two bio-sand filters were constructed according to the Centre for Affordable Water and Sanitation Technology guidelines (CAWST 2008). Plastic buckets 25ml in volume

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were used and were packed with multi-media filter material. The multi-media filter bed consisted of fine sand of 0.3mm diameter and 250mm deep; sand of 0.95mm diameter and 750mm deep; gravel of 7mm diameter and 50mm deep. The South African National Standard (SANS 3001) were used to determine the particle size and grading in order to achieve the required particle size distribution of the filter media. Dewatering of the filter between charges is avoided by a vertical discharge tube that rises from 2-7 cm above the height of the filter media. The elevated outlet allows the media to remain saturated after a charge has been filtered and when water is no longer flowing from the outlet (Fig. 1). The design parameters of the filter are summarised in Table 1.

Table 1: Summary of the design values used for the two filters (BSF & BSFC)

Design parameter	Unit	Recommended Value	Applied Value
Media depth	m	0.3-0.5	0.3
Supernatant depth	mm	50	50
Surface area	m <sup>2</sup>	0.06	0.071
Effective size	mm	0.15-0.40	0.35
Coefficient of uniformity		1.5 to 3	2.64
Filtration velocity (in clean filter	m/hr	0.10 to 0.6	0.17-0.63
bed)			
Inflow rate	m <sup>3</sup> /hr	0.03 to 0.04	0.04

The filtration cycle of a biofilter is made-up of resting time (6-24hrs) and a maximum filtration time of about 2hrs (Fewster et al., 2004). The biological treatment occurs during the resting time and after this period the filter bed is drained. In this study the raw surface water was fed into the filter once a day and the resting time and filtration time were 24hrs and 2hr respectively. The filtered water was collected in a 5 litre vessel for laboratory analysis. The average inflow rate was measured from noting the start time of filtration and the time periods at which the level of the water in the receiving vessel changed by 1 litre.

The superficial velocity  $(v_s)$  is related to the surface area of the filter and is normally used in filtration computations and is also equivalent to the hydraulic surface loading divided by the surface area of the filter. For BSF, the inflow rate is not constant since the water is only poured once for a filter cycle and hence the infiltration velocity decreases with time from the start to end of cycle.

## Nitrate and Carbon source dosage

A stock solution of ammonium nitrate ( $NH_4NO_3$ ) of concentration of 190g/l was dosed to both filters (BSF and BSFC), and to achieve a dose of 25mg/l in the 25l filter volume, 3.33ml of the stock solution was required. The ethanol was applied only to BSFC at C/N ratios of 1.1 and 1.8. With a molar mass of 46g/mol of ethanol ( $C_2H_5OH$ ) the carbon equivalent in the ethanol was 24g/mol (52.2%). Therefore, at a nitrate dose of 25mg/l and C/N ratio of 1.1 the dosage of carbon as ethanol in a 25l biosand filter was 7.45ml of carbon as ethanol. Similarly, at C/N ratio of 1.8, the required dose of carbon as ethanol was12.1ml. The surface loading of  $NO_3$ -N was calculated by multiply the concentration of nitrate with the superficial velocity ( $g/m^2$ .d).

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## 173 Filter maturation

The de-nitrification in biosand filters is biological and take place under a fixed film growth process whereby the bacteria develop on the surface of the sand media. For the smooth operation of the biosand filter, the water level was maintained at 50mm above the fine sand. The maturation period for the full development of the biological layer and acclimatising of the microorganisms to ethanol and NO<sub>3</sub>-N environment was 3 weeks. The operating temperatures of the filters varied between 19°C and 20°C and were not controlled in order to simulate the actual operating conditions of a biosand filter at household level.

## **Sample Collection and Analysis**

Sampling bottles were washed with distilled water before and after sampling. The samples were collected at the inlet and outlet of the two biosand filters in 500ml Erlenmeyer flasks and stored in a refrigerator at 4°C and analysed within 1 hour. The frequency of sample collection was once a week after the 12 hr resting time.

 The pH and dissolved oxygen (DO) were measured using a pH meter Model HACH HQ30D (FLEXI Model). The instrument was calibrated and measurements conducted in accordance with the Standard Method. The nitrate was measured by Spectroquant Nitrate Photometrical Test Method using Merck Spectrophotometer PHARO100 and the results were reported as NO<sub>3</sub>-N in mg/l. The carbon source which was ethanol was measured as Chemical Oxygen Demand (COD) by the MERCK Spectroquant TR 320 Digester (Spectroquant COD Cell Test method). The samples were digested in tubes containing a mixture of chromic and sulphuric acid with silver sulphate as a catalyst. After digestion samples were cooled and read on the Spectroquant PHARO100 Spectrophotometer. The COD test was carried out mainly to determine the amount of ethanol as a carbon source in the source water before and after the filtration process.

## **RESULTS AND DISCUSSIONS**

# Flow rates

Initial flow rates in the control filter BSF started from 0.04m³/hr and declined to 0.03m³/hr by end of experiment, whilst in BSFC which received carbon source the flow rate reduced from 0.04m³/hr to 0.01m³/hr (Fig. 2). The reduction in flow rates was comparable to studies conducted on Bio-sand filters by Kubare & Haarhoff et al., (2010) and Kennedy et al., (2012). The declining in the filtration rate was due to filter clogging and was substantial when the biological layer was fully mature. The reduction in the flow rate was more pronounced in the filter dosed with an external carbon source (BSFC) compared to one without carbon (BSF). Therefore, there was more growth of the biomass in the biofilter with an external carbon source due to the favourable environment conducive for growth of heterotrophic bacteria. Conventional surface cleaning will not remove the biomass at the bottom layers. Consequently, a household would require more filters to meet the daily water demand as well as increasing the resting period in BSFC to reduce excessive growth of biomass. Overall, the filtration velocity ranged from 0.17m/hr to 0.63m/hr and typical filtration rates for BSF range from 0.16 to 1.1m/hr (Elliot et al. 2008; Kubare & Hannoff 2010).

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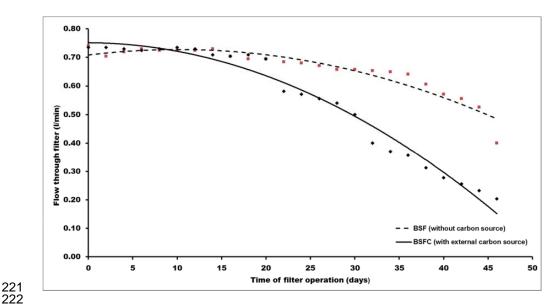


Figure 2: Variation of flow rates in the filters with and without carborn source

## Changes in pH and DO

The pH and DO are important physiochemical parameters in the removal of nitrates in BSF. There was no significant change in the pH of the influent and effluent water for both filters (BSF and BSFC). Overall, there was a slight decrease in pH fron 8.6 to 6.8 and such a pH range would favour the de-nitrification process since maximum denitrification rates are acheived at pH range of 7 to 8.5. Whereas for pH values smaller than 6 and larger than 8.5 would result in a sharp decrease in the de-nitrification activities. However pH may increase during de-nitrification because the reduction of nitrate to gaseous nitrogen with organic substrate as an electron donor results in the production of carbon dioxide and oxygen hydroxide (OH), which may react to form a bicarbonate (HCO<sub>3</sub>) and carbonate (CO<sub>3</sub><sup>2</sup>) (Drtil et al. 1995; Wang et al., 1995). With regards to water quality guidelines, the pH values were within the acceptable South African (2015) guideline limits of 5.0 to 9.7.

The overall reduction of DO in the filter with an external carbon source was 65% with average inflow and outflow concentrations of 8.23mg/l and 2.94mg/l respectively. However, the reduction in dissolved oxygen was less in the filter without an external carborn source (50%). The reduction in the DO is due to the oxgen demand by aerobic and nitrifying bacteria at the top of the filter bed.

# **Nitrate Removal Rates**

The nitrate removal mechanisms during heterotrophic de-nitrification are bacterial respiration and bacterial synthesis (Mohseni-Bandp et al., 2013). The de-nitification will take place at the bottom of the filter bed where there is less oxygen (anoxic conditions). William and Beresford (1986) concluded that nitrification and denitrification happen simultaneously in zones where there are short distances between the aerobic and anaerobic zones. The same scenario is depicted in biosand filters due to the short filtration length of apprximately 0.3-0.5m (Elliot, CWAST).

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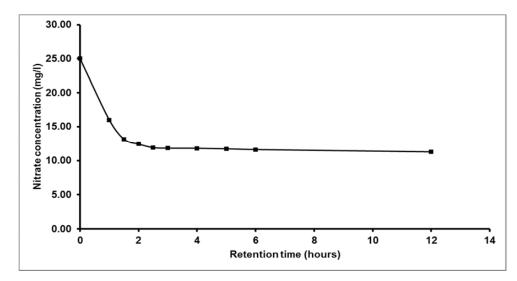


Heterotrophic bacteria need organic carbon as the electron donor and as the source of carbon, whilst getting their oxygen by removing bound oxygen from nitrate ( $NO_3^-$ ) which is in the water being treated and the nitrate acts as the electron acceptor. As a result of this process, the removal rate of nitrates in the filter without external carbon source (BSF) was 30%±0.04 (Table 2) and Mahlangu et al. (2011) reported a rate of 37% in similar filters. In the filter with an external carbon source (BSFC) the nitrate removal rate was 44%±0.03 at C/N ration of 1.1 and 53%±0.02 at C/N ratio of 1.8. Overall the nitrate removal rate was higher with the use of an external carbon source at higher C/N ratio of 1.8 (Table 3). However, the effluent nitrate concentration was between 16 to 19mg/l but still above the recommended guideline values in potable water.

The failure to achieve effluent nitrate guideline values even though pH was optimum could be due to high DO. Optimum de-nitrification occurs under anoxic conditions when oxygen levels are depleted (low redox) and nitrate becomes the primary oxygen source for heterotrophic bacteria. In general it has been observed that a dissolved oxygen concentration of more than 0.2mg/l reduces the rate of de-nitrification significantly (Jorgensen and Sorensen, 1984). High levels of dissolved oxygen were recorded ranging between 2.9mg/l to 8.2mg/l and hence were higher than the optimum values for de-nitrification.

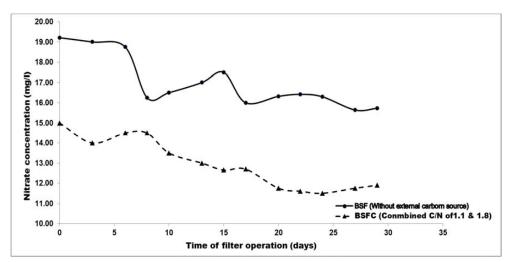
Reducing the DO concentration in biosand filter will enhance the nitrate removal efficiency but will compromise the aerobic microbial activity at the top layer. A feasible alternative will be to increase the filter depth so as to create an anoxic zone at the bottom or to increase the resting period of the filter. Bio-sand filters are designed with a filtration time of 2 hrs and resting period of 12 to 24 hours (CAWST, 2007; Elliott et al., 2008). The resting time provides the contact time for microbial removal and denitrification processes and thus a long resting time will be desirable from this perspective. However, too long a resting period may reduce the viability of the biological layer because the survival of the microorganisms relies on the periodic inflow of source water for nutrients (Baumgartner et al., 2007). Additionally, too long a resting period will reduce the water production rate and thus fail to satisfy household water requirements. Therefore careful selection of the resting period is vital in order to balance these competing objectives. In this study a resting time of 12hrs was used and nitrate concentrations measured during this period showed a rapid removal rate during the first 1.5 hrs and no significat removal thereafter (Fig. 3). Therefore, increasing the resting period more that the 12 hours will not have any sigificance in the nitrate removal. However, results for the entire operational period indicates low removal at the begining (40%) and thereafter the rate increased to 53%, and this illustrates the importance of maturation period. The variation of nitrate concentrations for the entire operational period are illustrated in Fig. 4.





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Figure 3: Reduction of nitrate relative to resting period in the filter with an external carbon source. Values of the nitrate are the average of the C/N ratio of 1.1 and 1.8.



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Figure 4: Variation of nitrtate concentrations for the entire operational period

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## **Denitrification Rate**

The denitrification rate was computed as:

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307  $R_{dn} = \frac{1}{t}(C_{in} - C_{out})$ 308 Where:309  $R_{dn}$  = denitrification rate (M/L<sup>3</sup>T)
310  $C_{in}$  = influent nitrate (M/L<sup>3</sup>)
311  $C_{out}$  = effluent nitrate (M/L<sup>3</sup>).

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The denitrification rate for BSF and BSFC were 3.66gNO<sub>3</sub>-N/m³.d and 5.44gNO<sub>3</sub>-N/m³.d respectively and these rates are lower than those reported by Aslan et al., (2007) in slow sand filters ranging between 8.1 and 29.2 gNO<sub>3</sub>-N/m³.day at filtration rates between 0.015 and 0.06 m/h.

Table 2: Nitrate removal effficiency at C/N=1.1 ant at influent nitrate concentration of 25 mg/l

Sampling interval (Days)	BSF (No external Carbon Source)		BSFC C/N=1.1 (With external carbon source)		
	Effluent Nitrate (mg/l)	Removal Efficiency	Effluent Nitrate (mg/l)	Removal Efficiency	
0	19.21	23%	15.00	40%	
2	19.00	24%	14.00	44%	
5	18.75	25%	14.50	42%	
7	16.25	35%	14.50	42%	
9	16.50	34%	13.50	46%	
12	17.00	32%	13.00	48%	
14	17.50	30%	12.65	49%	

Table 3: Nitrate removal effficiency at C/N=1.8 ant at iinfluent nitrate concentration of 25 mg/l

Sampli ng interval (Days)	BSF (No external Carbon Source)		BSFC (With external carbon source) C/N=1.8		
	Effluent Nitrate (mg/l)	Removal Efficiency	Effluent Nitrate (mg/l)	Removal Efficiency	
17	16.00	36%	12.70	49%	
20	16.32	35%	11.75	53%	
22	16.42	34%	11.60	54%	
24	16.30	35%	11.50	54%	
27	15.64	37%	11.75	53%	
29	15.73	37%	11.90	52%	

### Residual COD in effluent

The residual ethanol measured as COD in filters with an external carbon source varied between 25mg to 35mg/l. Overall, the removal efficiency of COD at C/N ratio of 1.1 and 1.8 was 89% and 90% respectively. There was rapid COD removal in the first 2 hours and became constant as the resting period increases and hence there is no significant benefit with longer resting periods. The same trend is depicted with nitrate removal which concluded that the de-nitrification process takes place in the first 2 hours when the COD is utilised in the process. However, the COD concentrations in the effluent were higher than the guideline values, and such high level of COD concentrations may be toxic to human health and increases disinfection by-product formation potential. This present a major health challenge in the use of an external

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carbon source in the removal of nitrates in potable water and there is a need to explore post-treatment methods to remove the residual carbon in biosand filters.

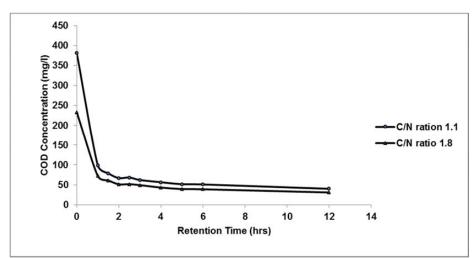


Figure 5: Reduction of COD relative to resting period in the filter with an external carbon source

Table 4: COD removal Efficiency at C/N =1 and C/N=1.8

Sampling interval (days)	Influent COD (mg/l)	Effluent COD (mg/l)	COD Removal Efficiency (%)	Sampling interval (days)	Influent COD (mg/l)	Effluent COD (mg/l)	COD Removal Efficiency (%)
	C/N = 1.1				C/N = 1.8		
0	233.52	26.85	88.50				
2	233.52	25.30	89.17				
5	233.52	25.61	89.03				
7	233.52	23.98	89.73				
9	233.52	24.77	89.39				
12	233.52	25.10	89.25				
14	233.52	26.36	88.71				
17	382.12	35.54	90.70	17	382.12	35.54	90.70
20	382.12	36.10	90.55	20	382.12	36.10	90.55
22	382.12	35.86	90.62	22	382.12	35.86	90.62
24	382.12	35.42	90.73	24	382.12	35.42	90.73
27	382.12	35.40	90.74	27	382.12	35.40	90.74
29	382.12	35.18	90.79	29	382.12	35.18	90.79

## **CONCLUSIONS**

Bio-sand filtration enhanced by an ethanol as an external carbon source has potential in the removal of nitrates in potable water at household level. The average nitrate removal efficiency in biosand filter with ethanol as an external carbon sourc at C/N

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ratios of 1.1 and 1.8 was 44% and 53% respectively. Although the nitrate concentration levels in effluent exceeded the recommended guidelines, the technology is capable of limiting nitrate in drinking water. Increasing the resting period more that than 12 hours will not have any sigificance in the nitrate and COD removal rates since these two processe take place in the first 2 hours. Dissolved Oxygen concentration in the effluent reduced significantly due to aerobic oxidation and nitrification which took place, simultaneously on the upper layer of the filter. The reduced DO low levels promoted hetertrophic de-nitrification at the bottom of the biosand filter. However, the DO levels were still above for optimum values for de-nitrification, and also the residual COD concentrations were above the water quality quidelines.

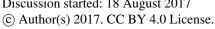
The flow rates reduced with time throughout the whole experiment due to the growth of the biological layer and clogging of the filter media and as a result the yield of the biofilter was reduced. The flow rate reduction was higher in the filter with an external carbon source and was substantial when the biological layer was fully mature. Overall, the study concluded that there is high potential in the use of POU filters enhenced with an external carbon source in the removal of nitrates through heterotrophic denitrification. The major challenge on the use of an external carbon source is the high residual COD concentration, which may pose a health risk.

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