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Water supply project feasibilities in fringe areas of Kolkata, India

K. Dutta Roy¹, B. Thakur², T. S. Konar³, and S. N. Chakrabarty⁴

¹Executive Engg., Kolkata Metropolitan Water and Sanitation Authority, Kolkata, India

²Senior Lecturer, Meghnad Saha Institute of Technology, Kolkata, India

³Graduate Student, Civil Engineering Dept., Jadavpur University, Kolkata, India

⁴Professor, Civil Engineering Dept., Jadavpur University, Kolkata, India

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Correspondence to: B. Thakur (b2981975@yahoo.co.in)

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Abstract

Water supply management to the peri-urban areas of the developing world is a complex task due to migration, infrastructure, paucity of fund etc. A cost-benefit methodology particularly suitable for the peri-urban areas has been developed for the city of Kolkata, India. The costs are estimated based on a neural network estimate. The water quality of the area is estimated from samples and a water quality index has been prepared. A questionnaire survey in the area has been conducted for relevant information like income, awareness and willingness to pay for safe drinking water. A factor analysis has been conducted for distinguishing the important factors of the survey and subsequent multiple regressions have been conducted for finding the relationships for the willingness to pay. A system dynamics model has been conducted to estimate the trend of increase of willingness to pay with the urbanizations in the peri-urban areas. A cost benefit analysis with the impact of time value of money has been executed. The risk and uncertainty of the project is investigated by Monte Carlos simulation and tornado diagrams. It has been found that the projects that are normally rejected in standard cost benefit analysis would be accepted if the impacts of urbanizations in the peri-urban areas are considered.

1 Introduction

The urban population is increasing at a rapid rate. In Asia, about 37% of the population was living in urban space in the year 2000. By 2030, the percentage is expected to reach about 54% which is equivalent to about 0.58% increase in each year. This is a significant departure from the trend of the same population group in 1950 when the annual average increase was only about 0.29% (Cohen, 2006). The trend is more pronounced in Kolkata (earlier called Calcutta). The population has increased from 11.02 million in 1991 to 13 million in 2001 indicating about 1.8% annual growth rate (Bannerjee and Das, 2006). Such an urbanization rate generated a tremendous pressure on civic facilities like water supply.

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World Health Organization, in a half a century old study, found that only 12% of urban population in India had house connections and 18% used public stand post for water. The remaining 70% has no facility for piped water (Dietrich and Henderson, 1963). The same study also found that in all most all of the 75 developing countries that were within the study scope, the water works developments are too slow to match the future needs. In a decade later, International Bank for Reconstruction and International Development Association (IBRD) started financing for the water supply system. Improvements like construction of mains, reservoirs, booster stations etc and small groundwater based systems in the fringe areas of Kolkata have been initiated (IBRD, 1973). Such ground water based small systems have become a norm for fringe areas of Kolkata for many years mainly because of lesser investments and smaller gestation periods. The maintenance cost for small ground water supply is relatively high (Dutta Roy and Chakravarti, 2006). In Kolkata, the ground water was found to be bacteria free but showed high concentrations of chlorides and other chemicals. Hardness and Iron (Fe), Chlorine (Cl), and sulfates increase from north to south side of Kolkata. Recent studies also show that the groundwater in some areas contains arsenic, lead, and cadmium in excess of the levels prescribed by the WHO for drinking water (KMC, 2006). Experience and awareness have increased the demand of piped surface water in the fringe area. The policy makers now shun away from ground water sources because of its detrimental impacts. For example, Asian Development bank (ADB) has recently sanctioned a 230 Mil US\$ loan for environmental improvement in Kolkata which includes construction of 8500 public stand posts etc. but ground water based small water supply systems have not been financed (KMC, 2007).

Many of the users in the fringe areas find the ground water unpalatable and regularly purchase surface water for drinking from unorganized sector. Llorente and Zerah (2003) found that in New Delhi, India the water suppliers in such areas enjoy a sizable market demand. Gessler and Brighu (2008) also found that in Jaipur, India the alternative providers supply good quality water in tankers or camel-powered cart in peripheral areas of the city.

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Water is traditionally supplied free of cost in Kolkata. Costs for the water supply are clubbed to the property tax (Ruet, 2002). Roy et al. (2003) reported that Kolkata Municipal Corporation (KMC) grants about 75% subsidy to water supply. As a result, the department is resource starved and soft international loans are availed for water supply infrastructures. However, international funding agencies require cost benefit analysis (CBA) and the authority is obligated to prepare explicit CBA as per best management practice. In typical CBA studies in India, the benefits are estimated from willingness to pay (WTP) studies conducted in the service area. It seems that the values obtained from such a WTP study in the fringe area would be inaccurate in the long run. The effect of rapid urbanization in the fringe areas should be integrated to the CBA study. In this paper, a CBA methodology with special reference to fringe area of the city that is under transition to piped water supply has been investigated.

2 Background

2.1 Previous study

The present authors investigated about the water supply network feasibilities in Kolkata in an earlier study (Dutta Roy et al., 2010). In the study, artificial neural network (ANN) have been trained for estimation of booster pumping station costs based on size, land rent and distance from the treatment plant. The benefit of the project is estimated from the willingness to pay survey conducted among beneficiaries within the Kolkata city. The costs and benefits occurred during the project life have been reduced to the present value for comparison. The variability of data has been studied with the help of Monte Carlo's simulation, uncertainty analysis and tornado diagram. The study has resulted in two key observations. Firstly, the net present value (NPV) of a booster pumping station is reduced to about zero at a distance of 20KM from the treatment plant. Secondly, the NPV is heavily dependent on the social discount and inflation rate. Accurate predictions of long term discount and inflation rate are really in the domain of

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the economists. Apart from it the study seems to have other latent issues in reference to its application to the fringe areas as explained in the following.

2.2 Issues

The monetary benefit from the project is based on WTP survey conducted on the beneficiaries. A number of WTP surveys have been conducted for water supply benefits in Kolkata (World Bank, 2001; Roy et al., 2003; Guha, 2007; Majumdar and Gupta, 2009). In addition, the authors also conducted a WTP survey in connection with the previous study (Dutta Roy et al., 2010). Each of these studies was conducted among the residents who had access to piped surface water. The dwellers living in the fringe areas of the city does not have access of the piped surface water as shown in the map in Fig. 3. They use ground water either from bore/tube wells or use the public stand posts served by ground water based small systems. Users suffer from the disadvantages like hardness, iron contents etc. Many of these users buy surface water for drinking/cooking. Ground water in some of the fringe areas contains harmful chemicals like arsenic (Bhattacharya et al., 1997), (Chatterjee et al., 1995). The user groups who suffer from water quality have already revealed their economic choices by purchasing water. The negative experience from hardness and iron content in ground water is likely to alter the WTP among those living in the fringe areas. The rising awareness about the harmful chemical in ground water may also change the WTP with time. The WTP of the previous studies were all conducted among the dwellers enjoying the piped surface water. The WTP in the fringe areas might be different which should be used in the CBA for water supply projects in these areas. Such issues are investigated in this study.

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3 **Background of the cost benefit analysis**

Efforts have been made in many countries for standardization of CBA. For example, Office of Management and Budget (OMB) in US has issued guidelines for CBA and risk assessment (Kopp et al., 1997). European Union started TECHNEAU, an integrated project funded by the European Commission that had a mandate to develop CBA for water supply system (Baffoe-Bonnie et al., 2006) and published guides (Florio, 2006). These studies do not specifically concentrate into urban-rural borderline where urbanization is taking place rapidly. The dwellers in fringe areas who purchase surface water reveal economic preferences that are not discernable in benefits assessed by willingness to pay (WTP) method conducted within Kolkata where water is supplied free of cost. The present paper has investigated the issues related to CBA of supplying piped surface water to these peri-urban users. A block diagram showing the methodology proposed for this CBA is presented in Fig. 1.

4 **Technical analysis**

4.1 **Study area**

The urbanization is taking place rapidly in the study area. Bhatta, Saraswati and Bandyopadhyay (2010) estimated the degree-of-freedom and degree-of-sprawl towards the analysis of urban growth in Kolkata from satellite images of 15 years. Bhatta (December 2009) analyzed the urban growth boundary for the city of Kolkata. The satellite image data were used to model the growth boundary of Kolkata for the years 2020 and 2035. The proposed model discouraged scattered development and increase in urban growth rate. They proposed a plan with polycentric urban blobs into a monocentric tract. In order to monitor the urban growth from remote sensing, researchers have attempted the concept of Shannon's entropy for determining the dispersion of built-up land growth. The concept is applied to several Indian cities like Pune (Sekhar, 2010),

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Hyderabad (Lata et al., 2010), Mangalore (Sudhira, Ramachandra and Jagadish, 2004) and Indore (Antony et al., 2007) with encouraging results. Bhatta (September 2009) applied Shannon's entropy model to Kolkata's urban growth. He finds higher growth rate of the built up area in the present study location that is the southern and eastern fringes of the city. The increase in built up area derived from satellite data by Bhatta (September 2009) is presented in Fig. 2. The population growth in Kolkata Municipal Corporation (KMC) and areas within the present study area as obtained from census records are also presented in Fig. 2. The Fig. 2 would indicate that population growth in the study area is increasing at a higher rate than that of the Kolkata city limits exerting a greater demand for drinking water services in the study area.

The study area is presented in the map in Fig. 3. The water supply scheme based on ground water based small systems in the peri-urban area of Kolkata was initiated in about three decades earlier (IBRD, 1973). The divisions of ground and surface water distribution as observed in 1995 were presented by Basu and Main (2001). The information has been used in Fig. 3. The population in the study area either uses ground water based small systems or individual bore wells. They are subjected to the detrimental effects of the ground water. Many of them purchase drinking water from vendors. Some of these areas are now in a transition stage for piped surface water from the two water works namely Garden Reach Water Works (GRWW) and Dakshin Roypur Water Works (DRWW). GRWW is administratively designated only for the city of Kolkata and DRWW serves the peri-urban areas. The details of the water works are presented in Table 1 and the locations of the waterworks and booster stations are shown in the map in Fig. 3.

4.2 Water quality

Water quality standards are not a fixed world wide standard. It varies significantly even in a country with the passage of time. In the setting of standards, competent agencies make political, economical, technical and scientific decisions about how the water will be used. For example, the US act requires reviewing the water quality standards in

every three years (US EPA, 2010). Indian standard (CPHEEO, 1999) has specified a series of requirements for potable water standards. Some of these that have been measured for this study are presented in the Table 2.

The Septic/Imhoff tank is the common form of sewage disposal method in the study area. Effluents are generally dispersed to ground through soak pits. Ground water up to a depth of about 15 m is not potable for contamination (Sahu and Sikder, 2009). In Kolkata, a Quaternary aquifer is sandwiched between two clay sequences (Sahu and Sikder, 2009). It starts from a depth of about 50 m. The depth of aquifer is about 300 m below which the bottom clay bed of Pliocene age occurs (Sikdar, Sarkar and Palchoudhury, 2001). The ground water is usually drawn from a depth of about 100 to 300 m depths. The Central Ground Water Board (CGWB, 2010) in India indicated salinity and arsenic as the major problem for ground water in Kolkata. Users in the study area depend on ground water because it is mostly safe from biological contamination which at least prevents the epidemics. However, many of the ground water samples in the study area suffer from other pollutants mentioned in the Table 2. The table would indicate that the users encounter considerable disadvantages in using such water. Expensive control methods that are not economically feasible in large scale in Kolkata are necessary to contain these pollutants. Users naturally have the preference for treated surface water.

The water quality as available to the users has been estimated from sample survey. Several types of potable water that are consumed in the peri-urban areas of Kolkata are collected. The sources and areas are detailed in Table 3. For each location five numbers of samples were tested for each of the potable water types.

Water quality testing

Water samples collected from the study area have been tested using standard laboratory procedures as per provisions of Standard Methods for the Examination of Water and Wastewater (APHA, 1995). A brief narrative about the testing procedure followed has been presented herein. A digital pH meter (Make: Eutech Instruments, Model:

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Eco Tester pH 2) is used for measuring the pH. The pH meter was calibrated with standard buffer solutions (pH-4.0, pH-7.0 and pH-9.2) prior to the measurement. Chloride content (as Cl^-) of the water samples are determined through titration using Mohr's method (Argentometric method) using 0.0141 N AgNO_3 (Silver Nitrate) solution as the titrant and K_2CrO_4 (Potassium Chromate) as the indicator. The end point of the titration is indicated by the first stable appearance of a reddish brown precipitate of Ag_2CrO_4 (Silver Chromate). The strength of the AgNO_3 titrant was standardized by titrating it against standard NaCl (Sodium Chloride) solution prior to titration of the water samples. A blank titration was also carried out with the DM water to remove any interference. Hardness (as CaCO_3) of the water samples is determined through titration using 0.02 N EDTA (Ethylenediaminetetraacetic acid) solution as the titrant and EBT (Eriochrome Black T) as the indicator. The end point of the titration is indicated by the change of colour from wine red to blue. Solution pH was kept around 10.0 during the titration by adding standard buffer solution of NH_4Cl (ammonium chloride). The strength of the EDTA titrant was standardized by titrating it against standard CaCO_3 (calcium carbonate) solution prior to titration of the water samples. A blank titration was also carried out with the DM water to remove any interference.

Iron concentration of the water samples is determined through colourimetric procedure using Phenanthroline method. Iron is brought in to ferrous state by boiling with HCl (hydrochloric acid) and $\text{NH}_2\text{OH}\cdot\text{HCl}$ (hydroxylamine) and allowed to react with $\text{C}_{12}\text{H}_8\text{N}_2\cdot\text{H}_2\text{O}$ (phenanthroline monohydrate) to form an orange-red complex which obeys Beer's law. Solution pH was kept at 2.9–3.5 during the titration by adding standard buffer solution of $\text{NH}_4\text{C}_2\text{H}_3\text{O}_2$ (ammonium acetate). The colourimetric measurement was carried out in an UV-Visible Recording Spectrophotometer (Make: VARAIAN, Model: 50-Bio) at a wavelength of 510 nm. The machine was calibrated with standard iron solutions prior to actual measurements. Arsenic concentration of the water samples are measured by AAS (Atomic Absorption Spectrophotometer – Make: VARIAN, Model: Spectra AA50). Fluoride concentration of the water samples is determined through colourimetric procedure using SPADNS method. Fluoride reacts with Acid

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zirconyl-SPADNS reagent dissociating a portion of it into colourless complex. The colour produced becomes lighter with the increase in Fluoride concentration. The colourimetric measurement was carried out in an UV-Visible Recording Spectrophotometer (Make: VARAIAN, Model: 50-Bio) at wavelength between 550–580 nm. The machine was calibrated with standard fluoride solutions prior to actual measurements. Total coliform is determined by multiple tube fermentation technique and results are reported as MPN (Most Probable Number) of organisms present. Ten replicate tubes each containing 10 ml sample are used for fermentation.

All gravimetric measurements are taken precisely in a digital balance (Make: METTLER-TOLEDO, Model: AB 135-S, Precision: 0.01 mg). AR-Grade chemicals (MERC) along with demineralized (DM) water were used for preparation of all the reagents and the blanks.

The results from the tests are presented in Table 3. The test results are compared with the Indian standard (BIS 10500-1991) which is also recommended by the Indian Central Public Health and Environmental Engineering Organization.

4.3 Water quality index

Water quality index (WQI) is a tool for simplifying the reporting of water quality data. Traditional reports similar to that of Table 1 consist of variable-by-variable report which is of value to experts. It supplies a wealth of data but could be overwhelming to non-chemists. For example, the Indian code for drinking water (BIS10500-1991) has more than 30 variables. The managers and decision makers may still wish to have a simpler measurement for overall representation of water quality. Indexing is a tool for such representations. There is no single measure that can describe overall water quality for a body of water. Although there is no globally accepted composite index of water quality, some authorities are using, aggregated water quality data in the development of water quality indices (UNEP, 2007). Most water quality indices rely on standardizing and weighted average of parameters according to their perceived importance to overall water quality (Sargaonkar and Deshpande, 2003).

CCME water quality index is a standardized and flexible index used in Canada and elsewhere. It was developed by the environment department of Canada after unifying a number of provincial indexes. The index is standardized against the provisions of the Indian drinking water standards (BIS10500-1991) for this study. The software (CCME, 2001) developed by the Canadian government is used. Similar indexing system has been used in Nagpur (Ramakrishnaiah et al., 2009) and Karanataka (Rajankar et al., 2009), India.

Water quality index estimate

The water quality index is based on the CCME method (CCME, 2001). There are three factors namely scope, frequency and amplitude in the index that are scaled to a range between 0 and 100. A brief definition of each factor is reproduced in Table 4.

The estimated WQI values ranges from 0 to 100 that are classified into five water quality categories namely excellent (95–100), good (80–94), fair (65–79), marginal (45–64) and poor (0–44). The WQI of the samples presented in Table 3 is estimated by the recommended software (CCME, 2001) and is reported in Table 3. The failing scores of the variables in the sample are based on the Indian standard (CPEEHO, 1999) which is also reported in Table 3.

5 Costing

Life cycle costing (LCC) is usually adopted for CBA. It is the total cost of ownership of an asset including its cost of acquisition, operation, maintenance, conversion, and/or de-commission (SAE, 1999). Since outflows occur over multiple time periods it is reduced to a present value using the standard principle of time value of money for standardized LCC methodology such as by US Government (Fuller and Petersen, 2002). The risk and uncertainty of cash flows are also considered in LCC (Emblemsvåg, 2003). A flow diagram of the life cycle cost of a typical water supply installation is presented in Fig. 4.

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The components of acquisition and annual sustenance costs are presented. In order to compress the stream of cash flows the economic adjustment components are considered. The components required for managing the variability of outcomes are also presented in Fig. 4.

The present value estimate that has been used in relation to this study is presented in Eq. (1).

$$C_p = \frac{\sum_{t=1}^L \frac{C_t(1+r)^t}{(1+i)^t}}{\sum_{t=1}^L Q_t} + \sum_{t=1}^L \frac{\frac{O_t(1+r)^t}{Q_t}}{(1+i)^t} \quad (1)$$

Where, C_p = Present value of cost in INR/KI, i = Discount rate at year t , C_t = Acquisition costs in INR incurred at year t , O_t = Sustenance costs in INR incurred at year t , Q_t = Quantity of water in KI generated at year t , L = Expected life of the facilities, i = Discount rate at year t and r = inflation rate at year t .

The inputs in Eq. (1) would require complete technical design, drawing and specifications for a specific installation and estimation of all operations of the facility which might be exhaustive, time consuming and sometimes infeasible for new projects. In order to circumvent such issues data driven models like artificial neural network (ANN) has been proposed for cost estimate (Adeli, 2001) (Boussabaine, 1996). The ANN is a mathematical model of theorized brain activity for processing an existing dataset (Zurada, 1992). Similar ANN techniques have been used in this case for estimating the cost of water supply.

5.1 Cost estimate

The costs of the facilities have been collected from the records of the concerned departments of GRWW in an earlier study. The authors (Dutta Roy et al., 2010) trained an ANN model among the existing cost data of GRWW network with the help of NeuroSolutions software (NeuroDimensions, 2009) and derived a cost model. The ANN

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cost model was based on three input factors namely land rent, serving population and the distance from the treatment plant. The model is applied to DRWW network and costs for the booster stations are estimated. The inputs as well as the ANN findings are presented in Table 5. The locations of the booster stations in the DRWW network as marked with the serial numbers in Table 5 have been presented in the map in Fig. 3.

6 Benefit

6.1 Benefit analysis

The benefit of the project is first estimated based on a willingness to pay (WTP) survey which is fairly common even in Kolkata (World Bank, 2001; Roy et al., 2003; Guha, 2007; Majumdar and Gupta, 2009; Dutta Roy et al., 2010). The consumer's spending capacity and experience and awareness about a product have an impact on the WTP. As a result of urbanizations the population in the study area is increasing rapidly. The newer residents are experiencing the ground water piped supply. The WTP for ground water piped supply in the peri-urban area is subjected to change because of actual experience and changing income profile among the population in the study area with the passage of time. In addition, the presence of harmful chemicals like arsenic etc. in the study area has been published in technical literature and media. The spending capacity, experience and awareness among the residents are increasing that is expected to change the WTP for ground water piped supply with the passage of time in the study area.

Turpie (2003), in a study about valuation of biodiversity in South Africa, found that the WTP for conservation have changed when the respondents were informed about the risks. Brouwer (2006) found that WTP for bathing water after a draught experience is not the same as before. In this case, the WTP for ground water in the peri-urban area is expected to change with time because of factors like increase in the average income profile for urbanization in the area, negative experience for hardness and iron

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content etc and additional awareness about harmful chemicals like arsenic in ground water. The expected change of WTP is modeled by dynamic system analysis.

6.2 WTP survey

A WTP survey was carried out for estimating the consumer willingness to pay for water among 201 samples in the study area. In addition to WTP, data was collected for 21 other variables likely to influence the consumer WTP. A correlation matrix for these variables (Pearson) is calculated assessing the underlying correlations between these variables. It is observed that two variables viz. WQI bathing/washing and difficulty for bathing/washing are poorly correlated (Pearson correlation coefficient <0.30) with other variables. These variables are omitted from further analysis as per usual practice (SPSS 2009).

6.2.1 Factor analysis

A principal component analysis followed by a varimax rotation carried out on remaining 18 variables using SPSS (2009) for determining the underlying factors of WTP. The analysis was run following the Kaiser's criterion of retaining the components with eigenvalues greater than one. The Kaiser's criterion seemed to be applicable here as the number of variables is less than 30 (SPSS 2009). The Kaiser-Meyer-Olkin measure of sampling adequacy is found to be 0.645 (>0.5) suggesting the samples are adequate. The Bartlett's Test of Sphericity is found to be highly significant ($p < 0.001$) suggesting Factor Analysis is appropriate (SPSS 2009) in this case. The analysis extracted five factors with eigenvalues greater than one which together accounted for 79.38% of the explained variance. The average communality after extraction has found to be 0.84 (>0.6) conforming to Kaiser's criterion. A Scree Plot is shown in Fig. 5.

The details of factor loadings, explained variance and eigenvalues of the five extracted factors are given in Table 6. The items with loadings greater than 0.6, are used

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to define five extracted factors. The variables for the regression of WTP are decided based on the extracted factors and are highlighted. A total of ten variables are selected.

6.2.2 Multiple regression

A multiple regression analysis has been conducted for WTP against ten explanatory variables selected from the factor analysis presented in Table 6. The mean and standard deviation for the selected variables as obtained from the survey is presented in Table 7. The R^2 for the regression is found to be 0.63 and the analysis of variance seems to be acceptable for such studies. The regression equation for WTP is presented in Eq. (2).

$$\begin{aligned} \text{WTP} = & 99.28 - 2.06 (\text{WQI_DC}) + 4.98 (\text{Fam_Size}) + 0.41 (\text{Avg_Age}) + \\ & 11.29 (\text{Avg_Edu}) + 0.02 (\text{PC_Incm}) + 0.22 (\text{Exp_DC}) - 9.16 (\text{Dfclt_DC}) \\ & + 0.02 (\text{Buy_Amt}) + 0.03 (\text{Buy_Mony}) - 25 (\text{Awr_Total}) \end{aligned} \tag{2}$$

The explanatory variables as shown in Table 7 are employed in Eq. (2) for conducting a Monte Carlo’s simulation. The monthly per family WTP is found to be 58.51 ± 67.43 . A sensitivity analysis has been carried out and the tornado diagrams for regression coefficients and Spearman rank correlation coefficients are presented in Figure-6.

The WTP is found to be most sensitive on per capita income (PC.Incm) and the least on awareness (Awr.Total). It has been hypnotized that the income would considerably increase in the peri-urban areas with the growth of urbanizations. Such increase should be considered in CBA particularly for the peri-urban areas where the effect would be pronounced to reflect the effect of the expansion of the city with time. Dynamic analysis technique is used to estimate the increase of such income.

6.3 Dynamic system analysis

The observed WTP is dependant upon variables like income which changes over time. As a result WTP is likely to change over time as well. System dynamics has been used to estimate the change of WTP with time.

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System dynamics (SD) analysis was first proposed by Forrester (1991). It is a methodology for studying complex feedback systems over time. It has been successfully used in many types of cases including urban planning. Vo, Chae and Olson (2010) found that SD model is suitable for urban management because it can handle different lags in economic, social and technical effects of large scale systems. Thus, it may help decision makers to avoid selecting alternatives apparently effective in the short term, but detrimental in the long term. Forrester who developed SD applied it to urban planning (Forrester, 1969) for its ability for long term effectiveness.

Winz et al. (2007) used SD for integrated water resources management. Winz (2005) also used SD for sustainable urban water systems. Chung et al. (2008) developed SD model for water and wastewater treatment plants. They simulated the model for twenty years and concluded that decentralized treatment facilities are more cost effective than centralized plants for both water and wastewater. Ho, Chang and Yang et al. (2007) chose SD after evaluating other optimizing models in connection with the conjunctive use of surface and subsurface water. They considered system expansions such as increasing new supply sources, and capacity expansion such as water treatment plant. They found that the SD model is a suitable methodology for constructing complex water resources models.

6.3.1 Dynamic analysis methods

There are number of commercial software that has been developed for SD model. In this case, iThink (iSee, 2009) has been used. It is a common SD application that has been used in several water quality books (Carlseen, 2004; Werick, 1994; Wurbs, 1994) etc. and used by institutes (UNESCO-IHE 2010) etc. Doerr (1996) studied the development of the Stella or iThink software system. Argent and Houghton (2001) used it for integrated land and water resources modeling. Palmer et al. (1997) used it for a water conflict resolution model in Korean river. Winz (2005) employed Stella for sustainable urban development with a focus on water in New Zealand. Rizzolia and Youngb (1997) used Stella for environmental decision making. Constanza and Ruth

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(1998) employed it for building environmental consensus. In the present study, iThink (iSee, 2009) software is used for analyzing system dynamics of income growth with time.

6.3.2 Urbanization and income growth

- 5 The growth of urbanization results in increase of built up areas. Such growth for peri-urban Kolkata has been estimate by Bhatta (September 2009) from satellite data and is presented in Fig. 2. The data has been combined with the geographical data for development of the regression based relationship presented in Eq. (3) and in Fig. 7a. The correlation (R^2) has been found to be 0.96.

$$10 \quad B = 6.5295e^{0.0333t_a} \quad (3)$$

Where B = built up area in km^2 and t_a = time in years since 1975.

- Using Eq. (3) present built-up area (at year 2010) is calculated as 20.9435 km^2 . The income and years of urbanization have been studied from the WTP survey data conducted in the eastern fringe of Kolkata viz. Mukundapur. A relationship is derived from regression as shown in Eq. (4) and in Fig. 7b. It has been observed that the average income of the population migrating to this area would increase as presented in Fig. 7b. The correlation (R^2) has been found to be 0.58. It is so expected because only progressively richer people could afford the price increase caused for rapid urbanization in this area.

$$20 \quad S = 1172e^{0.2074t_i} \quad (4)$$

Where S = Average per capita income and t_i = time in years of urbanization.

6.3.3 Dynamic simulation

- The increase in built up area and per capita income in the fringes of Kolkata with the years of urbanization is evaluated over a period of 20 years in an iThink (2009) dynamic simulation model as shown in Fig. 8.

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The iThink software automatically simulates a time dependent dynamic system, comprising different input variables (shown in Fig. 8) based on assigned relationships. The model component relationships are presented in Table 8.

The model is simulated for ten time steps i.e. for 20 years and the average per capita incomes are presented in Table 9 and increase of income with time is presented in Fig. 7.

The regression relationship between incomes with time is presented in Eq. (13). The correlation (R^2) is found to be 0.92.

$$PC_Incm = 2947.5e^{0.1094t} \quad (13)$$

Where, PC_Incm = Instantaneous Average Per Capita Income of the System (INR/Capita/Month), t = Year of Calculation (time in year from recent).

6.4 WTP estimate

The questionnaire survey conducted in the eastern fringes of Kolkata viz. Mukundapur provided the opportunity to estimate the WTP by regressing against the income as presented in Eq. (14) and based on Eq. (13) it is rewritten as in Eq. (15). The daily per capita WTP is presented in Eq. (16). Since the daily per capita water demand is 150 lpcd (CPHEOO, 1999) the WTP per KI of water may be written as in Eq. (17).

$$WTP_m = 0.0211PC_Incm + 6.1137 \quad (14)$$

$$WTP_m = 0.0211 \left(2947.5e^{0.1094t} \right) + 6.1137 \quad (15)$$

$$WTP_d = \frac{WTP_m \times 12}{365 \times Fam_Size} \quad (16)$$

$$WTP_0 = \frac{(WTP_d) \times 1000}{150} \quad (17)$$

Where, WTP_m = Monthly per Family WTP (INR/Family/Month) and PC_Incm = Monthly per family income (INR/Family/Month), WTP_0 = WTP per Kilolitre of water (INR/kl).

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7 Benefit cost comparison

7.1 Net present value

The present value of WTP after correction for inflation for each kl of water is estimated from Eq. (17) and presented in Eq. (18). The net present value of WTP for each KI is presented in Eq. (19).

$$WTP_p = \frac{\sum_{T=1}^{75} \frac{WTP_0(1+r)^T}{(1+i)^T}}{T} \quad (18)$$

$$NPV = WTP_p - C_p \quad (19)$$

Where, WTP_p = Present value of WTP for each KI of water, r = average inflation rate, T = Expected life of the water treatment facility for which CBA is conducted, C_p = present value of cost of the facility for which CBA is conducted.

The WTP_p (Eq. 18) is simulated over time as presented in Fig. 10a. The graphs in Fig. 10b–d show the variation of NPV of WTP for each KI of water (Eq. 19) over time. The range between \pm three standard deviation has been also presented as bands in Fig. 10. The costs of the facilities for booster stations in Kamalgazi, Begampur and Krishnamohanpur are obtained from the ANN model presented in Table 5.

7.2 Risk

The inputs for estimation of WTP and NPV are stochastic in nature. The common procedure for investigating such issues in CBA is by means of Monte Carlo's simulations (MCS). For example, Whittington et al. (2004) used MCS in Kathmandu, Nepal and Zhang (2009) used it in Singapore for water supply projects. MCS has been recommended by the regulatory authorities in counties like Canada (2007) and EU (Jasper, 2008). In MCS analysis, possible inputs depending upon its probability distributions are provided to the model and the resultant outcomes are noted. After numerous

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operations, the mean and standard deviations of the outcomes are assumed as the distributions of the possible outcome values.

The risks in inputs like the family size (Fam_Size), rate of inflation (r), rate of discount (i) are presented as triangular distribution in Table 10. The present value of ANN generated cost of supply (C_p) as shown in Table 5 are also presented. Monte Carlo's simulations consisting of 10 000 iterations have been conducted by Palisade @Risk (DTS, 2009) software for estimating the standard deviation of NPV as shown in Fig. 11. The results of the Eq. (19) for typical three cases have been presented in Fig. 11 as examples.

7.3 Uncertainty

The common procedure for investigating uncertainty in a CBA is done by means of sensitivity analysis (ADB, 1999; Baffoe-Bonnie, 2006; Canada, 2007; Florio, 2006). It identifies the critical parameters for the solution and determines systematically the influence of parameter variations on the solution (Fellin et al., 2005). The guidelines of OECD (2006) also specified sensitivity study for CBA. Sensitivity analysis is an iterative process. The variables in Equation 19 are changed systematically and the resultant NPV is recorded. After numerous iterations, the data set would provide an idea about the input which has the largest effect on the NPV. The 'Toprank' software of the Palisade system (DTS, 2009) has been used to conduct the sensitivity analysis. The results of the sensitivity analysis are presented in the tornado diagram in Fig. 12. The inflation and discount rate is found to be much more significant than other factors like project cost (C_p). The power series relationship of inflation (r) and discount (i) in the NTP and the long time periods of 75 years have made it much more sensitive than other factors like the project cost.

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8 Results and discussions

A previous CBA study (Dutta Roy et al., 2010) conducted for the city of Kolkata has been extended for the peri-urban areas in this article. The block diagram presented in Figure1 has been followed for a CBA methodology suitable to peri-urban areas. The urbanization is taking place rapidly in the suburbs of Kolkata as evidenced from Fig. 2 derived from the satellite data (Bhatta, September 2009). The infrastructure for drinking water supply is mostly inadequate in the peri-urban areas. The map in Fig. 3 developed from a study by Basu and Main (2001) would indicate that the piped surface water supply is available only in the city areas. In the peri-urban areas, people mostly depend on ground water from individually owned bore wells or surface water from ponds. Only pockets of suburbs have small groundwater based piped water systems installed mainly by external grants (IBRD, 1973). The water samples from these sources have been tested according to standard methods (APHA, 1995), compared with the national standards (CEEPHO, 1999) and presented in Table 3.

It may be noted from Table 3 that water quality of the fringe areas are inadequate in a number criteria prescribed in the Indian standards (CEEPHO, 1999). WQI has therefore been used for simplifying the quality issues about variable by variable comparisons. Canadian WQI method (CCME, 2001) as presented in Table 4 and calibrated to Indian standards has been used in the present case. It is observed in Table 3 that the WQI of water from pond or well in the fringe areas are of “poor” quality. The WQI has been found to be improved to about “fair” quality for ground water. The piped surface water obtained from the treatment plant on the other hand has the WQI of above 90 indicating “good” quality. The users in the suburbs would naturally prefer piped surface water. In order to explore the consumers’ preferences a water quality survey has been conducted in the peri-urban areas. It has been found from the survey that a large percentage of people actually buy water and most have a positive WTP for treated surface water.

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In a previous study (Dutta Roy et al., 2010) a neural network model has been developed for rapid estimation of water supply network costs. The model has been employed for estimating costs of the booster pumping stations in the DRWW network as shown in Table 5.

The benefit of the water supply has been initially estimated from a WTP survey. Varieties of queries about the water supply have been added to the survey. A factor analysis presented in Table 6 indicates that five factors would account for about 80% of the variance. The remaining queries that are grouped in the flatter portion of the scree plot in Fig. 5 contribute less to the variance. The multiple regression study with ten explanatory variables as selected in Table 6 has been conducted and the regression relationship is presented in Eq. (2). The mean and standard deviation for the selected explanatory variables as obtained from the survey is presented in Table 7. These statistics have been used for conducting a Monte Carlo's simulation of the Equation 2. A sensitivity analysis has been carried out and a tornado diagram has been presented in Fig. 6. The WTP is found to be most sensitive on per capita income of the surveyed population.

The high dependence of WTP on income has created a special problem for the peri-urban areas. Since urbanizations are taking place rapidly in peri-urban areas as indicated in Fig. 2 the per capita income are also increasing faster in these areas. The CBA of a water supply project based on the present per capita income of the surveyed population may not be appropriate only after a few years because of increased per capita income caused by urbanizations. Since the water supply projects are usually for a long period of time the increase of per capita income with urbanizations should also be included for a realistic CBA.

A SD analysis has been conducted for ascertaining the increase of per capita income with time and the results have been plotted in Fig. 9. The regression relationship between income and time as derived from SD analysis is presented in Eq. (13). It may be noted that this per capita income rise in the peri-urban area is from the result of urbanization because progressively richer people could afford housing in these areas.

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The income rise as a result of inflation is a different issue and has been addressed in the present value estimate analysis in Eq. (18). The cost derived from Table 5 and the benefits derived from Eq. (13) has been used in Eq. (19) for estimating the net present value and the results are presented in Fig. 10.

It may be noted in Fig. 10 that NPV for each of the booster stations in DRWW network namely Kamalgazi, Begampur and Krishnamohanpur are negative at the start. These projects should normally be rejected in a standard CBA. However, the NPV for each of these projects become positive after about 5–10 years because of increase of per capita income in the area as a result of urbanization as shown in Fig. 10. The impact of urbanizations should therefore be included in the CBA conducted in the peri-urban areas.

A Monte Carlos simulation of Eq. (19) has been conducted for estimating the risks associated with the NPV. Triangular distribution of risks for input variables like inflation and interest rates have been considered. The mean and standard deviation of the NPV for the three numbers booster pumping stations in the DRWW network are presented in Table 12. The distributions of the NPV for three typical cases have been presented in Fig. 11. The uncertainty associated with the NPV is investigated in the Tornado diagram in Fig. 12. The inflation and discount rate is found to be most significant. The power relationship of inflation (r) and discount (i) with NPV have made these more sensitive than other factors.

9 Conclusions

The development of the infrastructures like water supply is a challenge for mega cities in the developing world where urbanizations are taking place rapidly. Continuous expansion of the city area makes the project implementations more complex. A standard CBA analysis may reject a number of proposals in the peri-urban areas in a developing country mainly because of lesser WTP at the time of CBA. However, the urbanization of the area might change the financial characteristics and the resultant WTP may be suf-

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ficient to affirm a project otherwise unviable. The factors of such urbanizations are not included in a CBA methodology recommended in developed countries like US (Kopp et al., 1997) or EU countries (Baffoe-Bonnie et al., 2006), (Florio, 2006) etc. probably because the developed world does not face such acute urbanization pressure in the peri-urban areas. However, the experience in Kolkata as presented in this article seems to show that these factors should be included in the peri-urban areas of the developing countries.

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Table 1. Water pumping stations in Kolkata and southern fringes.

Supply	Sl	Station Name	Tributary pop. (000)	Tank (MG)	Supply (MGD)
GRWW	1	GRWW Ph1			
	2	GRWW Ph2	579.6	NA	60.00
	3	Behala	550.9	3.5	9.00
	4	Mahestala	236.1	5.0	10.00
	5	Ranikuthi	241.3	3.5	7.00
	6	Garfa	185.9	3.5	7.00
	7	Kalighat	319.7	4.0	8.00
	8	Bansdroni	122.7	2.0	4.00
	9	Pujali	34.7	0.70	2.10
Total			2630.9	22.20	107.10
DRWW		DRWW Direct	757.8	NA	21
	10	Kamagazi	341.8	1.0	3
	11	Begampur	419.7	1.0	3
	12	Krishnamohanpur	47.4	1.0	3
Total			1993.9		30

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Table 2. Typical pollutants and its impact on potable water.

Pollutants	Acceptable limit	Cause for rejection limit	Effect	Typical control methods
pH	7.0–8.5	< 6.5 or >9.2	Low pH: Corrosion High pH: Deposits	Add chemicals
Hardness (mg/l, CaCO ₃)	200	600	Scaling Soap scums	Ion exchanger, reverse osmosis
Chloride (mg/l, Cl)	200	1000	Taste, corrosion, blood pressure	Activated carbon reverse osmosis
Fluoride (mg/l, F)	1.0	1.5	Teeth and bone decay	Ion exchanger, reverse osmosis
Iron (mg/l as Fe)	0.1	1.0	Colour, sediment, taste, health effects	Oxidizing filter, Zeolite coagulant
Arsenic (mg/l, As)	0.01	0.05	Skin and nervous system toxicity	Ion exchanger, reverse osmosis
Coliform (no./100 ml)	0	0	Gastrointestinal illness	Chlorination Ultraviolet

Table 3. Water quality observed in southern fringes of Kolkata.

Source	Area	pH	Chloride (mg/l)	Hardness (mg/l ₃)	Iron (mg/l)	Arsenic (mg/l)	Fluoride (mg/l)	Coliform (N/100 ml)	WQI
Piped surface water	W. Fringe Behala	7.04 (±0.07)	15.65 (±1.55)	4.80 (±2.77)	BDL	BDL	BDL	0	91.10
	E. Fringe Mukunda.	7.03 (±0.05)	15.98 (±1.20)	5.00 (±1.58)	BDL	BDL	BDL	0	91.10 Good
Pond	W. Fringe Behala	7.02 (±0.07)	58.60 (±6.02)	20.20 (±1.64)	BDL	BDL	BDL	3900 (±291.6)	39.00
	E. Fringe Mukunda.	7.03 (±0.09)	90.60 (±5.77)	29.20 (±2.39)	BDL	BDL	BDL	3800 (±158.1)	38.60 Poor
Open Well	W. Fringe Behala	7.01 (±0.02)	23.41 (±3.30)	108.00 (±8.37)	0.14 (±0.05)	BDL (±)	BDL (±)	1760 (±167.3)	35.70
	E. Fringe Mukunda.	7.06 (±0.02)	44.01 (±2.47)	158.00 (±8.37)	0.16 (±0.05)	BDL	BDL	1620 (±178.9)	38.80 Poor
Piped Ground water	W. Fringe Behala	NA	NA	NA	NA	NA	NA	NA	NA
	E. Fringe Mukunda.	7.36 (±0.03)	173.30 (±5.32)	318.60 (±5.59)	0.88 (±0.08)	BDL	BDL	0	60.80 Marg.
Tube well	W. Fringe Behala	7.53 (±0.10)	48.21 (±5.91)	173.20 (±5.07)	0.70 (±0.27)	BDL	BDL	0	70.90 Fair
	E. Fringe Mukunda.	7.52 (±0.08)	295.54 (±8.99)	413.20 (±7.79)	1.26 (±0.05)	BDL	BDL	0	48.60 Marg.
CPH EEO	Acceptable Limit	7.0–8.5	200	200	0.10	0.01	1.00	0	

Note: Standard deviations where applicable are presented under the mean in parenthesis.

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Table 4. CCME WQI expressions.

Factors	Description	Expression
Factor 1, (F1) Scope	Represents the extent of water quality guideline non-compliance over the time	$F_1 = \left[\frac{\text{No. of failed variables}}{\text{Total no. of variables}} \times 100 \right]$
Factor 2, (F2) Frequency	Represents the percentage of individual tests that do not meet objectives (“failed tests”)	$F_2 = \left[\frac{\text{No. of failed tests}}{\text{Total no. of tests}} \times 100 \right]$
Factor 3, (F3) Amplitude	Represents the amount by which failed test values do not meet their objectives.	$F_3 = \left[\frac{\text{nse}}{0.01\text{nse}+0.01} \right]$ Where $\text{nse} = \frac{\sum_{i=1}^n \text{excursion}}{\text{No. of tests}}$
WQI	CCME WQI is then calculated as:	$100 - \left[\frac{\sqrt{F_1^2 + F_2^2 + F_3^2}}{1.732} \right]$

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Table 5. ANN Results of the DRWW Network.

Sl	Station	Tributary pop. (000)	Land rent (INR)	Supply (KI/day)	Distance (KM)	ANN Cost (INR/KI)
10	Kamalgazi	341.803	608986	51270.45	57.92	5.2990 ± 0.2161
11	Begampur	419.718	341032	62957.70	42.48	4.8360 ± 0.2166
12	Krishnamohanpur	474.514	243594	71177.10	41.15	4.7970 ± 0.2056

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Table 6. Varimax rotated factor loadings for five-factor solutions.

Factors	Variable Symbol	Variables	Selection	Factor Load
Factor 1 Variance = 19.61 Eigenvalue = 5.62	Aw_r_Total	Total Awareness	Yes	0.972
	Aw_r_Iron	Awareness about Iron Contamination		0.906
	Aw_r_Arsn	Awareness about Arsenic Contamination		0.904
	Aw_r_Path	Awareness about Pathogenic Contamination		0.778
Factor 2 Variance = 19.25 Eigenvalue = 3.98	Avg_Edu	Average Education Level of the Family	Yes	0.830
	WTP	Willingness to Pay		0.828
	PC_Incm	Monthly Per Capita Income of the Family		0.796
	Total_Incm	Monthly Total Income of the Family		0.780
	Low_Edu	Lowest Education Level in the Family		0.695
Factor 3 Variance = 18.79 Eigenvalue = 2.25	High_Edu	Highest Education Level in the Family	Yes	0.691
	WQI_DC	Water Quality Index for Drinking and Cooking		0.844
	Buy_Amt	Amount of Water Bought Per Month		0.822
	Buy_Mony	Money Spent for Buying Water Per Month		0.743
	Dfclt_DC	Degree of Difficulty with Drinking / Cooking		−0.720
Factor 4 Variance = 11.03 Eigenvalue = 1.76	Exp_DC	Years of Using the Drinking and Cooking water	Yes	−0.718
	High_Age	Highest Age in the Family		0.855
	Fam_Size	Number of Members in the family		0.684
Factor 5 Variance = 10.70 Eigenvalue = 1.46	Low_Age	Lowest Age in the Family	Yes	0.908
	Avg_Age	Average Age of the Family		0.708

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Table 7. Statistics of the variables selected for multiple regression.

Variables	Mean	Standard Deviation
WQI_DC	82.90	9.97
Fam_Size	4.55	1.60
Avg_Age	36.42	7.86
Avg_Edu	4.87	1.64
PC_Incm	2145.35	2883.28
Exp_DC	9.87	8.91
Dfclt_DC	1.93	1.24
Buy_Amt	306.24	274.54
Buy_Mony	125.22	110.42
Awr_Total	7.63	2.54
WTP	58.51	84.27

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Table 8. Relationships within the iThink Model.

Model Component Description	Relationships	Eq. No.
Built up Area in km ² (<i>B</i>)	$B = 6.5295e^{0.0333(2t_i+35)}$	(5)
The rate of increase in built up area	$\frac{dB}{dt_i} = 0.4349e^{0.0333(2t_i+35)}$	(6)
Percentage of city population migrating to fringe areas (<i>P</i>)	$P = B \times \frac{100}{20.9435}$	(7)
Migrants increase rate	$\frac{dP}{dt_i} = \frac{dB}{dt_i} \times \frac{100}{20.9435}$	(8)
By Eqs. (6) and (8)	$\frac{dP}{dt_i} = 2.0764e^{0.0333(2t_i+35)}$	(9)
Per capita income of any first group from today (<i>S</i>)	$S = 1172e^{0.2074(2t_i+9)}$	(10)
Average per capita income increase rate of first group with respect to iThink time step	$\frac{dS}{dt_i} = 486.1456e^{0.2074(2t_i+9)}$	(11)
Instantaneous Average per Capita Income of the System (INR/Capita/Month)	$\frac{\sum_{iI} i\% \times i\% \text{INCOME}}{\sum_{iI} i\%}, \quad i \in [I, III, V, \dots, XXIX]$	(12)

Note: All relations are with respect to iThink time scale (1 time step = 2 years). Where, *P* = population migrating to the fringe area in percentage of the present population, *B* = Built up area in km², *S* = Average per capita income of the migrants.

Table 9. iThink Model results.

iThink Time Steps	Time Years	Average per Capita Income (INR/Capita/Month)
0	0	4207.73
1	2	4244.40
2	4	4440.84
3	6	4869.12
4	8	5638.99
5	10	6917.04
6	12	8955.88
7	14	12 138.16
8	16	17 043.27
9	18	24 548.19
10	20	35 980.16

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Table 10. Parameters for WTP and NPV simulation.

Symbol	Unit	Low	Average	High
Fam_Size	Number	2.00	4.55	8.00
R	%	5.00	6.00	7.00
I	%	6.00	7.00	8.00
C_p	INR/kl	Mean	Standard Deviation	
	Kamalgazi	5.2990	± 0.2161	
	Begampur	4.8360	± 0.2166	
	Krishnamohanpur	4.7970	± 0.2056	

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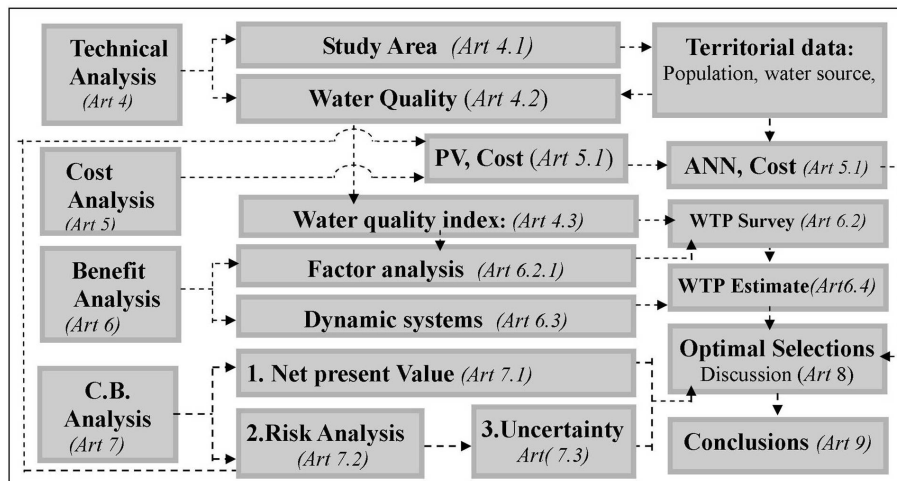


Fig. 1. Block diagram for the CBA.

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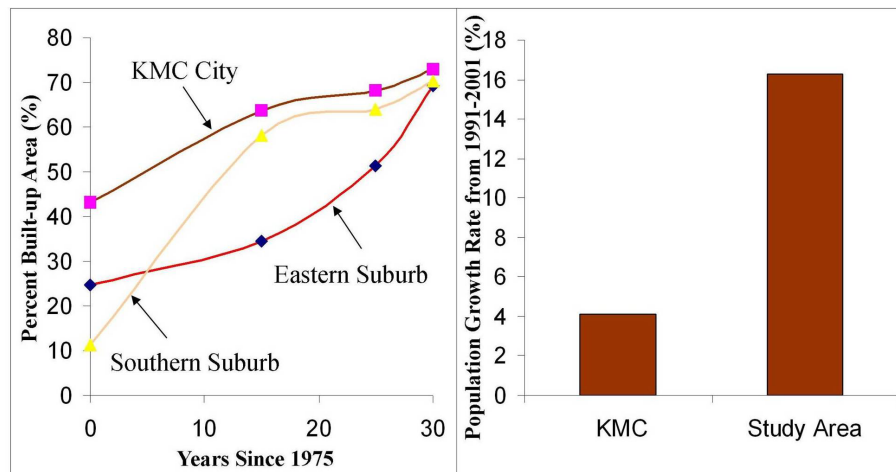


Fig. 2. Urban growths in fringes of Kolkata.

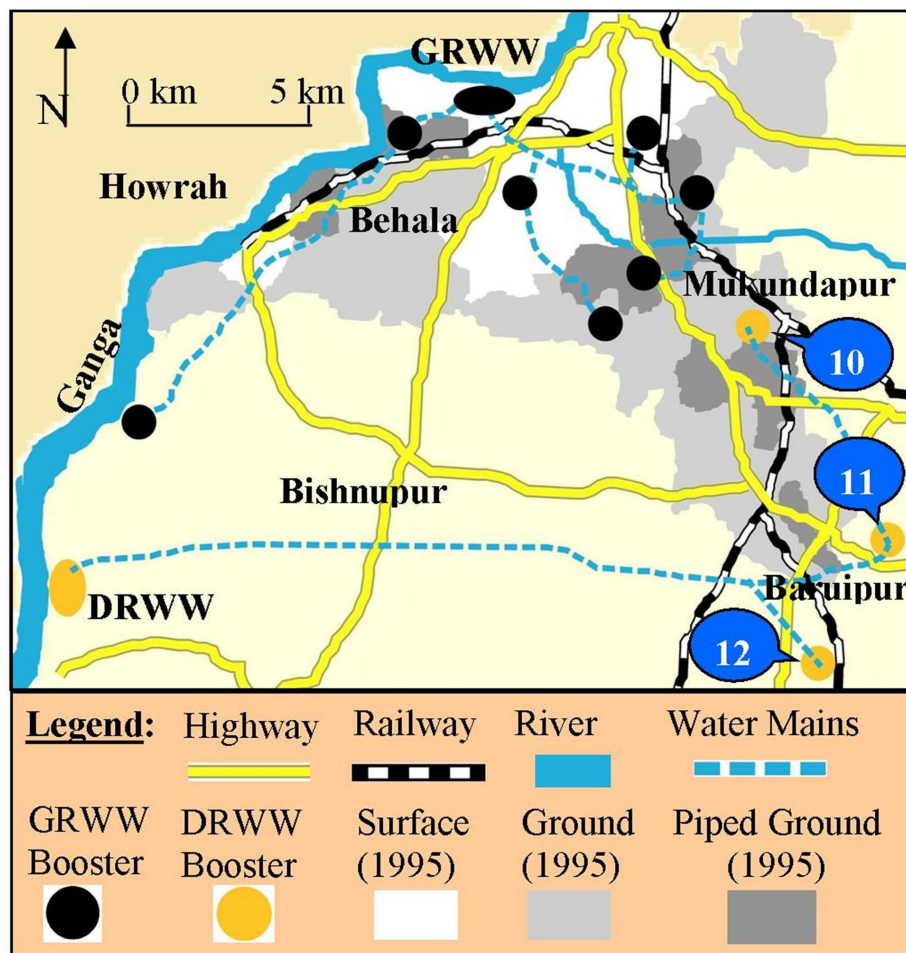


Fig. 3. Study area in Kolkata, India.

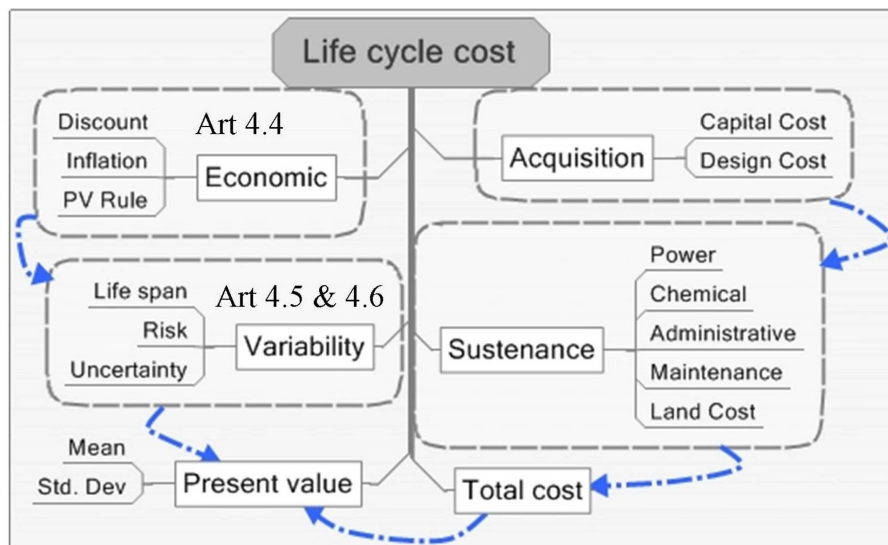


Fig. 4. Block diagram for cost estimate.

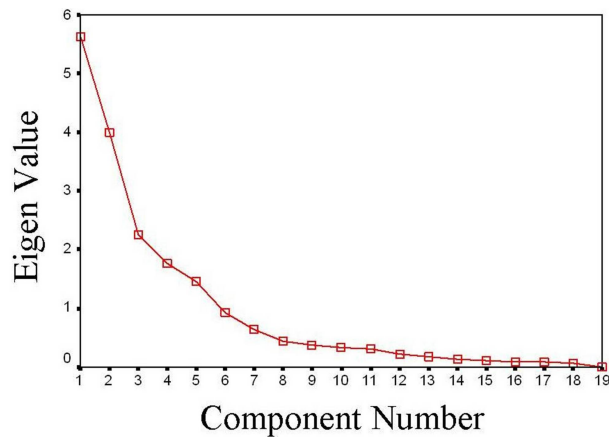


Fig. 5. The scree plot.

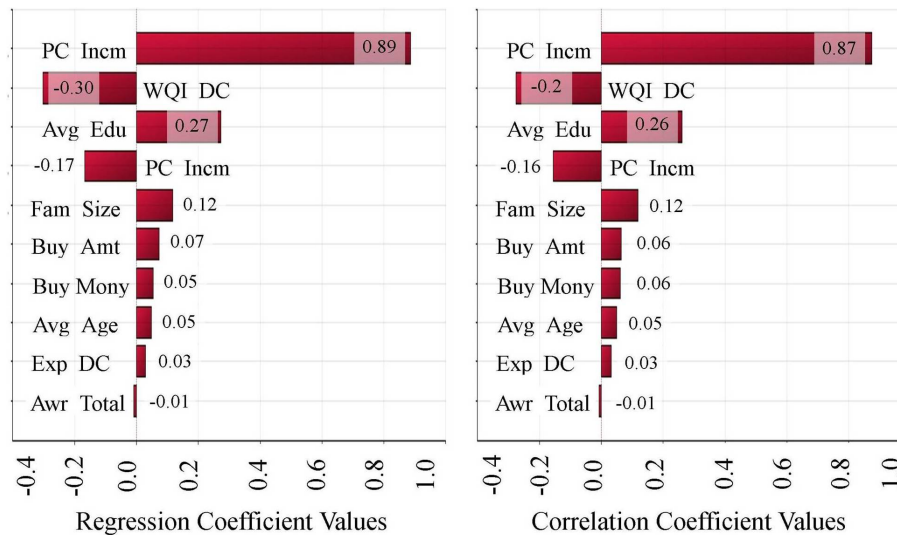


Fig. 6. Tornado diagrams for sensitivity analysis of WTP.

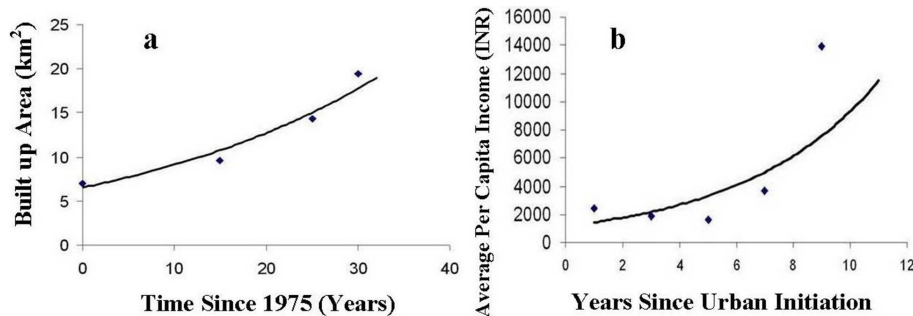


Fig. 7. Trends of income and urban growth.

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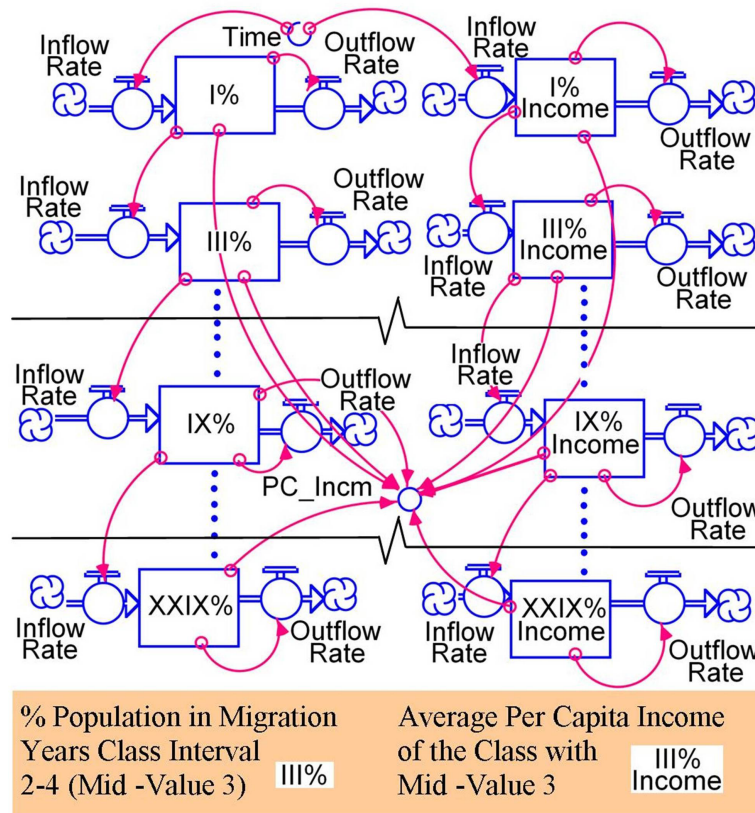


Fig. 8. Dynamic simulation model in iThink.

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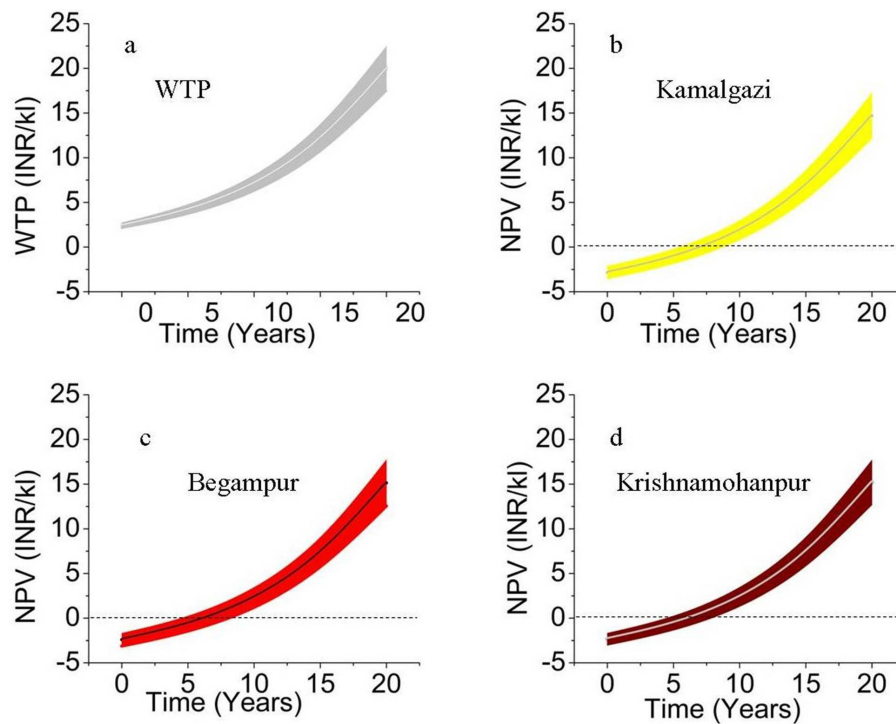


Fig. 10. Variation of WTP and NPV with time.

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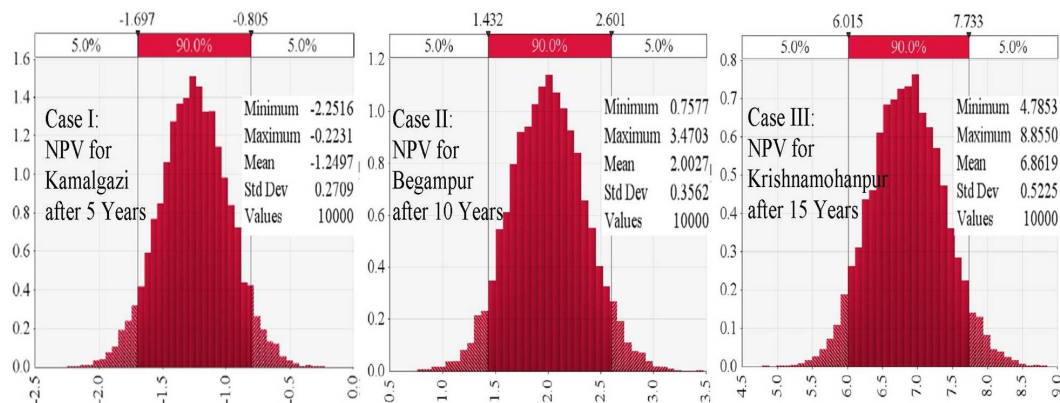


Fig. 11. Distributions observed in typical simulation runs for NPV.

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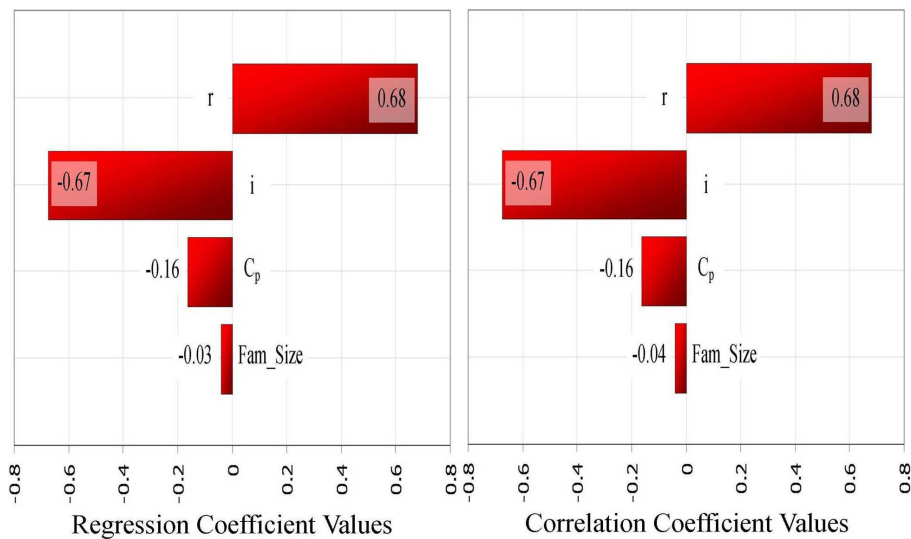


Fig. 12. Tornado diagrams for sensitivity analysis of NPV.